DESIGN DATA

1. Building code:IBC 2018 as adopted and amended by the local agencies having jurisdiction

2. Assumed bearing capacity=1000 psf

Ground

1

Risk category : ii
Wind speed : 98 mph
Wind exposure : C
Site class : D
Ss : 0.637
S1 : 0.204
SDs : 0.548
SD1 : 0.298
Seismic analysis method : Light framed wood wall sheathed with wood panel, r = 6.5

3. Load Criteria

Roof Dead Load :15 psf
Roof Live Load :20 psf
Ground Snow Load : 5 psf

4. Contractor shall verify all dimensions and conditions prior to installation, and notify Engineer or Architect of Record of any discrepancies.

5. Contractor is responsible for the design,adequacy, and safety of erection bracing, shoring, temporary supports etc. The contractor is responsible for the stability of structure prior to the application of all structural elements and finish materials on construction documents.

GENERAL NOTES

All material and workmanship shall be performed in accordance with IBC 2018

1. Contractor shall verify all dimensions and conditions at the job site prior to ordering any material and/or commencing work and shall report any discrepancies to the structural engineer of record.
2. Contractor shall provide barricades and pedestrian protection as required by state and local codes.
3. Omissions or conflicts between various elements of the architectural and structural drawings,notes and details shall be brought to the attention of the engineer and resolved before proceeding with the work.
4. All dimensions shall take precedence over scale shown on plans,sections and details.
5. If there is any discrepancy on the drawings,the most stringent condition governs
6. Where no details shown or noted on the drawings,the details shall be same as for other similar work.
7. Notes and details on drawings shall take precedence over these general notes.
8. Opening,pockets,sleeves,etc shall not be placed in slabs,beams,walls,columns and footings unless specifically detailed on the structural drawings.
9. Construction materials shall be spread out if placed on framed floors or roof. loads shall not exceed design live loads for each particular level. provide adequate shoring and bracing if load exceeds design live load or where structure has not attained design strength.
10. This set of drawings represent the finished structure,method of construction not necessary indicated, the contractor shall provide all measures necessary to protect the structure,workers and other persons during construction. such measures shall include but not be limited to bracing,shoring ,scaffolding, etc.
11. For one story construction,existing or new,the continuous footing dimension shall satisfy 12" minimum into lowest adjacent grade and 12" minimum in width. general contractor is responsible to verify in field and notify eor immediately if there is any discrepancy.
12. General contractor shall review all structural drawings before bidding and construction and verify all existing conditions of the existing structures to match the information shown on structural plans. general contractor shall notify or immediately if there is any discrepancy.
13. Temporary erection bracing and shoring shall be provided as required on all beams,walls,etc.,adequate provide full structural stability and safety. bracing shall not be removed until the elements are fully connected and are capable of supporting the design loading.
14. Contractor agrees that he shall assume sole and complete responsibility for job site conditions during course of construction of this project,including safety of all persons and property.that this requirement shall apply continuously and not be limited to normal working hours. and that the contractor shall defend indemnify and hold the owner and the engineer harmless from any and any liability ,real or alleged in connection with the performance of work on this project,excepting for liability arising from the sole negligence of the owner or the engineer.

STRUCTURAL LUMBER

1. All rough lumber used in the work shall be of the following grade of Spruce Pine Fir #1/2 with the base design values complying with wood production association (WPA) grading specifications,unless otherwise noted on plans
   2x,4x Plate,Rafter,Joists,Beam,Stud,Posts: SPF#2
   6x,8x Beams,Posts: SPF#STRUCTURAL
2. The following wood structural members are of Weyerhaeuser product,but products from any manufacturer is acceptable providing equivalent or better properties are presented with current approved ICC ES evaluation report
   LSL (TimberStrand): fb=2325 psi,fv=400 psi,e=1.55x10^6 psi
   PSL(Parallam),LVL(Mircollam): fb=2900 psi,fv=290psi,e=2.0x10^6 psi
3. Glu-Lam beam shall be DF/DF,24F-V4: fb_bottom=2400 psi,fb_top=1850 psi,fv=265 psi,e=1.8x10^6 psi, with standard camber on roof beams except cantilever end(u.n.o). all cantilever ends and floor beams shall have zero camber u.n.o. all beams shall be fabricated using waterproof glue.fabrication and handling of Glue-Lam beams shall follow ANSI/AITC A 190.1. standard beams to bear legible APA-ENS or AITC gradestamp. an APA-EWS CRAN AITC certificate of conformance for glued-laminated members should be submitted to the field inspector prior to installation.
4. Sheathing shall be DFPA graded stamped,type structural i,or CDX(240),exterior glue unless noted otherwise.15/32" and 19/32" thick sheathing shall have index no.32/16". 23/32" sheathing shall have index no 48/24.
5. Roof framing,sheathing and nailing shall be inspected prior to placing of roofing materials.
6. Provide metal washers for all bolts and nuts bearing on wood.
7. All bolt holes in wood members shall be a minimum of 1/32 inch to a maximum of 1/16 inch larger than the bolt diameter (per NDS 11.1.2). wood members with holes not meeting the above criteria shall be removed and replaced by the contractor at his own expense.
8. All lumber in contact with concrete that is in contact with the ground, shall be redwood or approved pressure treated wood.
9. Individual pieces of sheathing shall not be less than 2'-0" in their least plan dimension, nor less than 8 square feet in area.
10. Mechanical ducts and equipment, sprinkler pipes, suspended ceiling may not be supported by or connected to the roof sub purlins or any 2x4 framing member.
11. Ceiling joists not shown on the drawings shall be per table 2308.10.2 of the IBC.
12. Provide double joist under parallel partition walls

NAILING SCHEDULE

1. JOIST TO SILL OR GIRDER, TOENAIL.....3-8d
2. BRIDGING TO JOIST, TOENAIL EACH END.....2-8d
3. 1" X 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL.....2-8d
4. WIDER THAN 1" X 6" SUBFLOOR TO EACH JOIST, FACE NAIL.....3-8d
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL.....2-16d
6. SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL.....16d (BOX) AT 16" O.C.
SOLE PLATE TO JOIST, AT BRACED WALL PANEL.....(3) 16d (BOX) PER 16"
7. TOP PLATE TO STUD, END NAIL.....2-16d
8. STUD TO SOLE PLATE.....4-8d, TOENAIL OR 2X SOLE: 2-16d 3X SOLE: 2-20d (BOX)
9. DOUBLE STUDS, FACE NAIL.....16d (BOX) AT 24" O.C.
10. DOUBLED TOP PLATES, FACE NAIL.....16d (BOX) AT 16" O.C.
DOUBLE TOP PLATES, LAP SPLICE.....8-16d
11. BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE, TOENAIL.....3-8d
12. RIM JOIST TO TOP PLATE, TOE NAIL.....8d AT 6" O.C.
13. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL.....2-16d
14. CONTINUOUS HEADER, TWO PIECES.....16d AT 16" O.C. ALONG EACH EDGE
15. CEILING JOISTS TO PLATE, TOENAIL.....3-8d
16. CONTINUOUS HEADER TO STUD, TOENAIL.....4-8d
17. CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL.....3-16d
18. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL.....3-16d
19. RAFTER TO PLATE, TOENAIL.....3-8d
20. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL.....2-8d
21. 1" X 8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL.....3-8d
22. WIDER THAN 1" X 8" SHEATHING TO EACH BEARING, FACE NAIL.....3-8d
23. BUILT-UP CORNER STUDS.....16d AT 24" O.C.
24. BUILT-UP GIRDER AND BEAMS.....20d AT 32" O.C. AT TOP AND BOTTOM AND STAGGERED 2-20d AT ENDS AND AT EACH SPLICE
25. 2" PLANKS.....2-16d AT EACH BEARING

NAILING & FASTENERS

1. All nails shall be common wire nails, unless noted otherwise.
2. Nailing other than roof or floor diaphragm shown on the drawings shall be in accordance with the uniform building code.
3. Nails for roof and floor sheathing shall be two and three eighths of an inch (2 3/8") long and shall have a minimum of one and five eighths of an inch (1 5/8") penetration into the framing members.
4. Where nail spacing is required to be less than 3" o.c., nails shall be staggered.
5. Placement and nailing of all sheathing must be inspected and approved before covering.
6. All post-installed anchors shall comply with "post-installed adhesive anchors" notes on this sheet.

FOUNDATION

1. The contractor shall establish building lines and proceed with the excavation of all footing as called for on the drawings.
2. Footings shall bear on natural undisturbed uniform earth or engineered compacted fill.
3. No reinforcing steel and no concrete shall be placed in any excavation prior to approval by the building department.
4. The top of all excavations shall be protected against heavy surcharge loads and from erosion due to rainfall or surface run-off during the entire construction period.
5. No soils report id provided. use following minimum code data: foundation design is based on the recommendations contained in (IBC). the below adjacent finished grade levels Allowable soil bearing pressure per IBC table 1806A.2 for:

   Pad footings ..... 1,500 psf
   Continuous footings.....1,500 psf

REINFORCING STEEL

1. All reinforcing steel, unless noted otherwise, shall confirm to A.S.T.M. specifications A615 and be intermediate grade 60 for bars #4 and greater, and grade 40 for bars less than #4 and all ties and dowels.
2. All reinforcement to be welded shall confirm to A.S.T.M. A706, unless noted otherwise.
3. Reinforcement marked continuous may be spliced by lapping 42 bar diameters in concrete and 48 bar diameters in masonry with 24 inch minimum lap in each case, unless otherwise noted on plans. all splices when detailed shall be located where shown on plans.
4. Reinforcing steel shall be accurately placed and secured in position with metal or concrete blocks, chars, spacers etc., before placing concrete.
5. Additional reinforcing required for erection of precast concrete shall be added per the contractors details.
6. Welded wire fabric for slabs on grade shall conform to A.S.T.M. A185, be in flat sheets and have min. lap of one parallel strand but not less than 6.
7. Minimum concrete cover for reinforcing steel shall be as follows,unless noted otherwise. Concrete below grade or in contact with soil: when cast against earth 3" , when formed 2" Walls above grade : exterior face 1 ½", interior face 1". Precast concrete elements : as detailed. Concrete slab on grade : reinforcing steel at center of slab, unless noted other wise.
8. Reinforcement detailing shall be in accordance with IBC section 1907.
9. Provide min. 1" or one bar dia. spacing , whichever is greater, between adjacent bars

CONCRETE

1. All concrete mix designs, conforming to IBC, shall be submitted to the engineer for review and approval before any concrete is placed. All concrete mixes shall have a minimum cement content of 5.25 sacks of cement per cubic yard of mix. All concrete mixes shall be certified by a concrete testing laboratory and signed by a local registered civil engineer.
2. Concrete shall have the following minimum 28-day compressive strength:
   - Standard: 2,500 psi concrete
   - Grade beams and caissons: 2,500 psi min.
   - Slab on grade and/or footings: 2,500 psi min.
   - All other concrete: 2,500 psi min.
   - The maximum concrete slump shall not exceed 4".
3. Grout under steel column base plates shall be "Por-Rok" or "Five Star Grout" or approved equal.
4. Portland cement shall be Type II or Type V where specified in the soils report, conforming to A.S.T.M. C150, and shall be tested. Aggregates shall be normal weight (145 pcf) u.n.o., conforming to A.S.T.M. C33, with concrete shrinkage characteristics of less than 0.050%. The water-cement ratio shall not exceed 0.52. Where specified, lightweight concrete for walls and floor slabs shall have an in-place density of 110 pcf. Lightweight aggregate shall be expanded shale or approved equal conforming to A.S.T.M. C330.
5. Concrete test samples shall be taken in accordance with A.S.T.M. and I.B.C. standards. Results of the 7 and 28 day tests shall be submitted to the engineer for his records. Slump tests are required for all test samples and must also be reported. Additionally, all lightweight concrete samples must have their in-place densities determined and reported.
6. Sides of footing pads may be poured against stable earth.
7. Trowel and retrowel slab for a smooth finish, with trowel marks showing wherever the concrete floor is exposed.
8. See architectural notes for colored or textured concrete.
9. Concrete formwork tolerances shall be in accordance with I.B.C. and A.C.I. standards.
10. All steel reinforcing, anchor bolts, dowels and other inserts shall be secured in position and inspected by the local building department inspector prior to the placing of any concrete.
11. Where noted, sound insulating cellular concrete shall be 100 pcf (13 psf at 1-1/2" thickness) for second floor fill over plywood sheathing. All areas are to be troweled and retroweled to a smooth, flat finish suitable for interior coverings. Provide waterproofing paper and mesh.
12. All concrete to be [cured/kept moist] for a minimum of 3 days.
13. Top of slab or footing under column base plates shall be finished smooth and level for full bearing.
14. Special inspection of concrete is not required where the specified 28-day compressive strength does not exceed 2,500 psi.

POST-INSTALLED ADHESIVE ANCHORS

1. adhesive: simpson strong-tie set-3g epoxy (icc-es esr-4057), or approved equal having a current icc-es report for cracked concrete (aci 355.4 / ac308). substitutions require written approval of the engineer of record before installation.
2. anchor rods: 5/8"ø continuous threaded rod, astm f1554 gr. 36 (or astm a193 b7), hot-dip galvanized. nut and plate washer at top of each rod.
3. minimum embedment into existing concrete footing / stem wall:

anchor type	rod diameter	min. embedment
shear wall sill anchor bolts (shear ab)	5/8"ø	7"
hold-down anchors (hdu2)	5/8"ø	10"

4. embedment shall be measured from top of existing concrete. anchors shall extend into footing / stem wall, not slab only.
5. existing concrete is assumed f'c = 2,500 psi min. contractor shall verify condition of existing concrete and notify engineer of any cracking, spalling, or voids at anchor locations.
6. scan for existing reinforcing (gpr or equal) before drilling. do not cut existing reinforcing. if conflict occurs, notify engineer for relocation.
7. minimum edge distance and spacing per esr-4057 and engineer's approval. hole depth shall not break through bottom of footing.
8. installation: drill hole diameter, hole cleaning (air-brush-air), base material temperature, gel and cure times shall follow the manufacturer's printed installation instructions (mpii) and esr-4057. do not load anchors before full cure. do not use expired adhesive. use manufacturer's mixing nozzle only.
9. installers shall be trained on the adhesive system. a copy of the mpii and esr-4057 shall be on site for the inspector.
10. special inspection of post-installed anchors is required. see special inspection for concrete construction.

SPECIAL INSPECTION

1. IN ADDITION TO THE REQUIREMENTS OF IBC SECTION 1704 (I.E., CONCRETE WITH 28-DAY COMPRESSIVE STRENGTH GREATER THAN 2,500 PSI; ANCHOR BOLTS AND POST-INSTALLED ANCHORS INSTALLED IN CONCRETE; AND REINFORCING STEEL), SPECIAL INSPECTION IS REQUIRED ON THE FOLLOWING ITEM(S) FOR THIS PROJECT. FIELD WELDING OF STEEL WORK, AND WELDING OF REINFORCING STEEL AND HIGH-STRENGTH BOLTS, ARE NOT APPLICABLE — NO STRUCTURAL STEEL OR FIELD WELDING OCCURS WITHIN THE SCOPE OF THIS PROJECT.
2. THE INSTALLATION OF POST-INSTALLED ANCHOR BOLTS, ADHESIVE (EPOXY) ROD ANCHORS, AND EXPANSION BOLTS INTO EXISTING CONCRETE — SEE "SPECIAL INSPECTION FOR CONCRETE CONSTRUCTION" BELOW.
3. SPECIAL INSPECTION FOR WOOD DIAPHRAGM/SHEAR WALL FASTENING IS NOT REQUIRED FOR THIS PROJECT — SEE BASIS BELOW. SPECIAL INSPECTION FOR REINFORCING STEEL PLACEMENT AND CONCRETE PLACEMENT IN THE NEW FOOTINGS AND SLAB-ON-GRADE IS NOT REQUIRED — EXEMPT UNDER IBC SECTION 1705.3, EXCEPTIONS 1 & 2 (ISOLATED/CONTINUOUS FOOTINGS OF LIGHT-FRAME CONSTRUCTION, ALLOWABLE SOIL BEARING PRESSURE NOT GREATER THAN 3,000 PSF).
4. SPECIAL INSPECTIONS SHALL BE PERFORMED BY ONE OR MORE REGISTERED DEPUTY (SPECIAL) INSPECTOR(S) APPROVED BY THE BUILDING OFFICIAL, RETAINED AND PAID FOR BY THE OWNER.
5. SITE VISITS CONDUCTED BY THE ENGINEER OF RECORD ARE FOR GENERAL OBSERVATION PURPOSES ONLY AND DO NOT CONSTITUTE SPECIAL INSPECTION.
6. TWO (2) PROPERLY COMPLETED AND SIGNED COPIES OF THE SPECIAL INSPECTION AGREEMENT, LIMITED TO THE ITEM(S) IDENTIFIED AS "REQUIRED" HEREIN (POST-INSTALLED ANCHORS), MUST BE SUBMITTED TO PERMIT SERVICES PRIOR TO ISSUANCE OF THE PERMIT.

SPECIAL INSPECTION FOR WOOD DIAPHRAGM & SHEAR WALL

	INSPECTION TASK	CONTINUOUS	PERIODICALLY	REFERENCE FOR CRITERIA (IBC SECTION)
HORIZONTAL WOOD DIAPHRAGMS / SHEAR WALLS WITH EDGE NAIL SPACING EQUAL TO OR CLOSER THAN 4"	SHEATHING TYP, GRADE THICKNESS, SPAN TATING, NAIL TYPE, SIZE & SPACING			SEC. 1705.12.2/1705.5.1

NOT REQUIRED FOR THIS PROJECT
SHEAR WALL SCHEDULE (SHEET S-03) SPECIFIES 8D @ 6" O.C. EDGE NAILING FOR ALL SHEAR WALLS (SW-1 – SW-4), WHICH EXCEEDS THE 4" O.C. SPACING THRESHOLD THAT TRIGGERS THIS SPECIAL INSPECTION.

SPECIAL INSPECTION FOR CONCRETE CONSTRUCTION

	INSPECTION TASK	CONTINUOUS	PERIODICALLY	REFERENCE FOR CRITERIA (IBC SECTION)
POST-INSTALLED ANCHORS (SHEAR WALL ANCHOR BOLTS AND HOLD-DOWN ANCHORS):	VERIFY ANCHOR TYPE, ROD DIAMETER, EMBEDMENT DEPTH, HOLE PREPARATION AND CLEANING, ADHESIVE EXPIRATION DATE, AND INSTALLATION PER MANUFACTURER'S INSTRUCTIONS AND ICC-ES ESR-4057.		✔	ACI 318-19 & 17.8.2, & 26.13: IBC TABLE 1705.3, ITEM

POST-INSTALLED ANCHOR INSPECTION ABOVE IS REQUIRED FOR THIS PROJECT
BY CONTRAST, REINFORCING STEEL PLACEMENT AND CONCRETE PLACEMENT FOR THE NEW FOOTINGS AND SLAB-ON-GRADE (FC' = 3,000 PSI) DO NOT REQUIRE SPECIAL INSPECTION — EXEMPT UNDER IBC SECTION 1705.3, EXCEPTIONS 1 & 2 (ISOLATED AND CONTINUOUS FOOTINGS OF LIGHT-FRAME CONSTRUCTION, PRESUMPTIVE ALLOWABLE SOIL BEARING PRESSURE OF 1,500 PSF, NOT GREATER THAN THE 3,000 PSF EXCEPTION THRESHOLD).

SPECIAL INSPECTION FOR EXISTING SITE SOIL CONDITION
(NOT REQUIRED IF ALLOWABLE SOIL BEARING PRESSURE IS LESS THAN 1,500 PSF, SOILS REPORT IS NOT REQUIRED BY THE BUILDING OFFICIAL, AND THERE IS NO CONTROLLED FILL PLACEMENT ON THE EXISTING BUILDING SITE)

	CONTINUOUS	PERIODICALLY
VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY PER SOIL REPORT		
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL PER SOILS REPORT		
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS PER SOIL REPORT		
VERIFY USE OF PROPER MATERIALS, DENSITIES & LIFT THICKNESS DURING PLACEMENT & COMPACTION F CONTROLLED FILL PER SOILS REPORT. (EXCEPTION: NOT REQUIRED FOR CONTROLLED FILL 12" OR LESS IN TOTAL DEPTH.)		
PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SIRE HAS BEEN PREPARED PROPERLY PER SOIL REPORT.		

NOT REQUIRED FOR THIS PROJECT
THIS PROJECT'S FOUNDATION DESIGN USES A PRESUMPTIVE ALLOWABLE SOIL BEARING PRESSURE OF 1,000 PSF (SEE STRUCTURAL GENERAL NOTES; SHEET S-00), NO GEOTECHNICAL/SOILS REPORT IS ON FILE OR REQUIRED BY THE BUILDING OFFICIAL, AND NO CONTROLLED FILL IS BEING PLACED — THE ADDITION BEARS ON UNDISTURBED NATIVE SOIL. SPECIAL INSPECTION OF EXISTING SITE SOIL CONDITION, PER THE CRITERIA BELOW, IS THEREFORE NOT REQUIRED FOR THIS PROJECT.

Donald Anderson

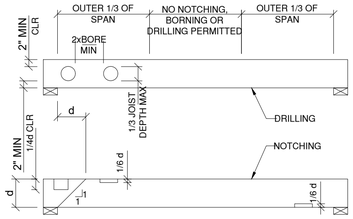
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SHEET INDEX

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GN-2	General Notes
S-1	Foundation Plan - Building 01
S-2	Framing Plan - Building 01
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S-4	Framing Plan - Building 02
SD-1	Structural Details
SD-1	Structural Details

ABBREVIATIONS

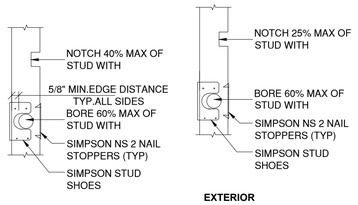
ABOVE - Above
AB - Anchor bolt
ACT - Asphalt concrete
AIR - Air conditioning
A.C.P. - Asphalt concr. paving
ADDL - Additional
A.F.F. - Above finish floor
ALUM - Aluminum
ALT - Alternate
ANOD - Anodized
ARCHT - Architectural
BEAM - Bottom of beam
BET - Between
BLDG - Building
BLKG - Blocking
BM - Beam
BN - Boundary nailing
BOT - Bottom
BOW - Bottom of wall
C - Channel
CANT - Cantilever
CG - Center of gravity
CIP - Cast in place concrete
CJ - Ceiling joist
CL - Center line
CLG - Ceiling
CLR - Clear
C.M.U. - Concrete masonry unit
COL - Column
COMPO - Composition
CONC - Concrete
CONN - Connection
CONT - Continuous
CONST - Construction
CORR - Corridor
CTR - Center
DBL - Double
DET - Detail
DF - Douglas FIR
DIAG - Diagonal
DIAPH - Diaphragm
DIA - Diameter
DIM - Dimension
DN - Down
DP - Deep
DS - Downspout
DWG - Drawings
(E) - Existing
EA - Each
EF - Each face
ELEV - Elevation
EMBED - Embedment, Embedded
EN - Edge nailing
EQ - Equal
ES - Each side
EW - Each way
EXIST - Existing
EXP - Expansion
EXT - Exterior
F.B. FB - Floor beam
FD - Floor drain
FDN - Foundation
FF - Finish floor
FG - Finish grade
FIN - Finish
F.J. FJ - Floor joist
FLG - Flange
FLR - Floor
F.O.C. - Face of concrete
F.O.M. - Face of masonry
F.O.S. - Face of stud
FW - Field nailing
FS - Far side
FT - Feet or foot
FTG - Footing
FV - Field verify
GALV - Galvanized
GA - Gauge
GI - Galvanized iron
GLB - GLU-LAM Beam
GYP BD - Gypsum board
"HUS" "U" - Hanger / Simpson
HDR - Header
HGR - Hanger
HK - Hook
HORIZ. - Horizontal
HP - High point
HT - Height
HS - High strength
HLDW - Holdown (see T.A.)
HVAC - Heating, Ventilating
HVAC - Air conditioning
IN - Inch
INFO - Information
INT - Interior
J.B. - Joist bearing
J.G. - Joist girder
JST - Joist
JT - Joint
KP - King post
KO - Knock out
L - Angle
LAT - Lateral
LDGR - Ledger
LG - Long
LLH - Long leg horizontal
LLV - Long leg vertical
LONGIT - Longitudinal
L.P. - Low point
LT - Light
MATL - Material
MAX - Maximum
M.B. - Machine bolt
MECH - Mechanical
MEZZ - Mezzanine
MFD - Manufactured
MFR - Manufacturer
MIN - Minimum
MISC - Miscellaneous
M.F.O - Metal framed opening
MLB - Microllam beam
M.P.H. - Miles per hour
MTL - Metal
(N) - New
N.I.C. - Not in contract
NO. - Number
N.S. - Nelson stud or near side
N.T.S. - Not to scale
O.C. - On center
OFF - Office
OPNG - Opening
OPP HD - Opposite hand
O.S.F. - Outside face
P.A. - Post above
P.B. - Post below
PAD - Pad footing
P.C. - Pile column
PEN - Penetration
PL - Plate of property line
PILAS - Pilaster
PLYWD - Plywood
PSF - Pounds per square foot
PSI - Pounds per square inch
P.T. - Pressure treated 2
R.D. - Roof drain
REBAR - Reinforcing bar
R.B. - Roof beam
REQD - Required
REINF - Reinforcing
REF - Reference
REV - Revision
R.R. - Roof rafters/Joists
RM - Room
R.O. - Rough opening
S.A. - Shear anchor bolt
SCH - Schedule
SEC - Section
SHTG - Sheathing
SHT - Sheet
SIM - Similar
S.J. - Sawcut joint
S.P. - Splice point
SPA - Spacing
SPECS - Specification
SSQ - Square
STAGG - Staggered
STD - Standard
STIFF - Stiffener
STL - Steel
S.S. - Select Structural
STRUCT - Structural
SYM - Symmetrical
T.A. - Tension anchor bolt
T & B - Top & Bottom
T & G - Tongue & Grove
TEMP. - Tempered
T.F. - Top of footing
T.A. - Top of beam
TG - Tapered girder
T.G. - Top of girder
THK - Thick
THRU - Through
T.L. - Top of ledger
T.N. - Top of nailer
T.O. - Top of
T.O.P. - Top of parapet
T.O.P. - Top of panel
T.O.S. - Top of steel
TOT - Total
TRANSV- Transverse
TRANSF- Transfer
TS - Tube steel
T.S. - Top of slab
T.W. - Top of wall
TYP - Typical
U.N.O. - Unless noted otherwise
UNDERP - Underpinned footing
VERT - Vertical
V.I.F. - Verify in field
W/ - With
WD - Wood
W - Wide flange
W/O - Without
W.P. - Work point
W.R. - Water resistant
WT - Weight
W.W.F. - Welded wire fabric
X - Extra strong
XX - Double extra strong



NOTES:
1. NOTCH MAX 1/4 d WITHIN d OF SUPPORT ON TOP SURFACE ONLY
2. NOTCH MAX 1/6 d WITHIN (OUTER 1/3 OF SPAN -d)
3. NO NOTCHING, DRILLING OR BORING SHALL BE ALLOWED WITHIN THE MIDDLE 1/3 OF THE SPAN, ON ALL SIDES OF THE MEMBER.

NOTCH/BORE % OF JOIST	2x6	2x8	2x10	2x12	2x14
16 2/3	7/8"	1 3/16"	1 1/2"	1 7/8"	2 1/8"
33 1/3	1 13/16"	2 3/8"	3"	3 3/4"	4 3/8"

TYP. DRILLING & NOTCHING OF WOOD JOIST | 1

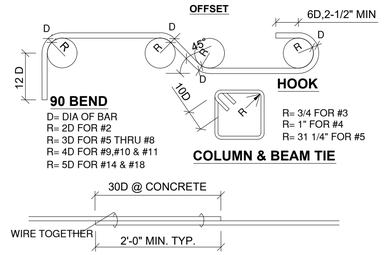


NOTES: NOTCHING & BEARING ARE NOT OCCUR IN THE SAME STUD

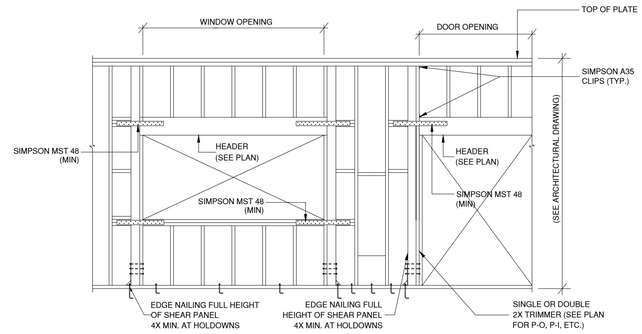
NOTCH/BORE %OF STUD	2x4	2x6
25%	7/8"	1 3/8"
40%	1 3/8"	2 3/8"
60%	2"	3 1/4"

TYP. DRILLING & NOTCHING OF WOOD STUD | 2

BAR NO	2	3	4	5	6	7	8	9	10	11	14	18	
BAR DIA	1/4"	3/8"	1/2"	5/8"	3/4"			1"	1 1/8"	1 1/4"	1 3/8"	1 3/4"	2 1/4"



TYPICAL REINFORCING BAR BEND & SPLICE | 3



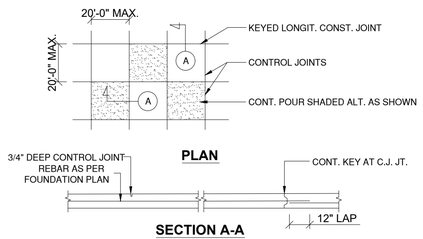
HEADER SHEDULE

CLEAR OPENING	HEADER SIZE	REMARKS
TO 4'-0"	4X4 OR 4X6	
4'-0" TO 6'-0"	4X6 OR 6X6	
6'-1" TO 8'-0"	4X8 OR 6X6	
8'-1" TO 10'-0"	4X10 OR 6X8	USE DBL. TRIMMERS
10'-1" TO 12'-0"	4X12 OR 6X10	USE DBL. TRIMMERS
12'-1" TO 14'-0"	4X14 OR 6X12	USE DBL. TRIMMERS
14'-1" TO 16'-0"	4X16 OR 6X14	USE DBL. TRIMMERS
16'-1" TO 18'-0"	6X16	USE DBL. TRIMMERS

NOTE: 1. USE ABOVE SCHEDULE SIZES ONLY WHERE SIZE IS NOT INDICATED ON PLANS OR DETAILS.
2. PROVIDE 2X TRIMMER AND KING STUD, TYPICAL U.O.N. NAILING PER NAILING SCHEDULE.

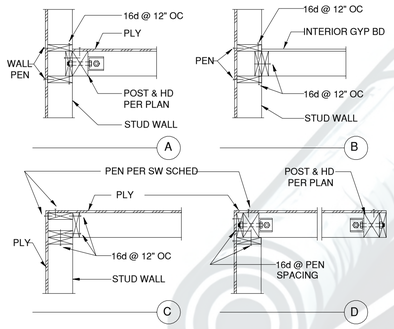
TYPICAL WALL ELEVATION AND HEADER SCHEDULE | 4

1.PANELS SHALL BE POURED IN LONG STRIPS OR SECTIONS HAVING A MAX WIDTH OF 20'-0".
2.PANELS SHALL BE SUBDIVIDED BY CONTROL JOINTS HAVING A MAX SPACING OF 20'-0".
3.REINF. SHALL BE SECURELY POSITIONED TO ENSURE IT'S PROPER FINAL LOCATION IN THE MIDDLE OF SLAB.
4.INITIAL AND CONTINUOUS CURBING SHALL BE MAINTAINED AS PER WRITTEN SPECIFICATIONS.

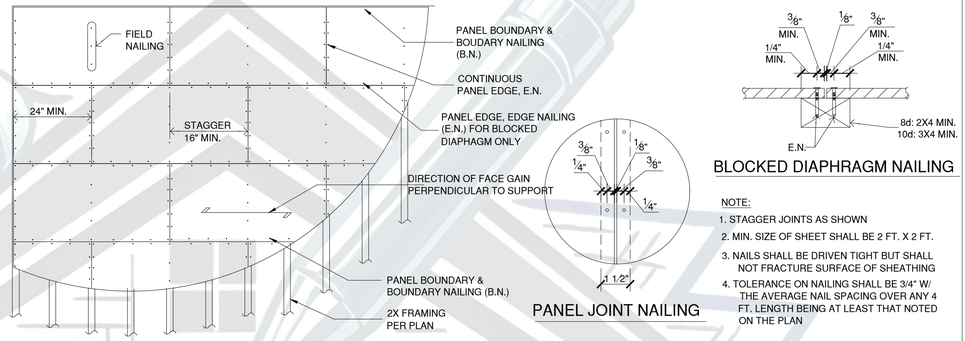


TYP. POURING SEQUENCE

TYPICAL SLAB ON GRADE | 5



TYP. STUD CONFIGURATION PLAN | 6



TYPICAL PANEL LAYOUT | 7

PANEL JOINT NAILING

BLOCKED DIAPHRAGM NAILING

NOTE:
1. STAGGER JOINTS AS SHOWN
2. MIN. SIZE OF SHEET SHALL BE 2 FT. X 2 FT.
3. NAILS SHALL BE DRIVEN TIGHT BUT SHALL NOT FRACTURE SURFACE OF SHEATHING
4. TOLERANCE ON NAILING SHALL BE 3/4" W/ THE AVERAGE NAIL SPACING OVER ANY 4 FT. LENGTH BEING AT LEAST THAT NOTED ON THE PLAN

8

TYPE	MATERIAL (BLOCKED)	FACES	PLYWOOD NAILS SPACING	SILL PLATE CONNECTION AT FLOORS,SEE DET. ON SHEET (IN PRE-DRILL HOLES)	BOT CONN. S.A. (SHEAR AB) 785 #/ANCHOR BOLT SEE FOUND.DETAILS	TOP PL CONN. A35 450# /ft SEE TYP. EAVE DETAILS AT ROOF	CBC SHEAR CAP. #/	SILL PL @ FOUNDATION PRESSURE TREATED
1	1/2" PLY ST1	1	10D @ 6"	20D @ 6"	5/8" @ 4'-0"	A35 @ 20"	245	2x
2	1/2" PLY ST1	1	10D @ 6"	20D @ 4"	5/8" @ 2'-8"	A35 @ 16"	340	3x
3	1/2" PLY ST1	1	10D @ 4"	20D @ 3"	5/8" @ 2'-0"	A35 @ 10"	510	3x
4	1/2" PLY ST1	1	10D @ 3"	1/4" DIAM x6" LAG @ 5" O.C.	5/8" @ 2'-0"	A35 @ 8"	665	3x
5	1/2" PLY ST1	1	10D @ 2"	1/4" DIAM x6" LAG @ 4" O.C.	5/8" @ 1'-4"	A35 @ 6"	870	3x
6	1/2" PLY ST1	2	10D @ 6"	1/4" DIAM x6" LAG @ 4" O.C.	5/8" @ 1'-4"	A35 @ 8"	940	3x
7	1/2" PLY ST1	2	10D @ 4"	1/4" DIAM x6" LAG @ 3" O.C.	5/8" @ 1'-0"	2-A35 @ 10"	1020	3x
8	1/2" PLY ST1	2	10D @ 3"	5/8" DIAM x6" LAG @ 6" O.C.	5/8" @ 1'-0"	2-A35 @ 8"	1330	3x

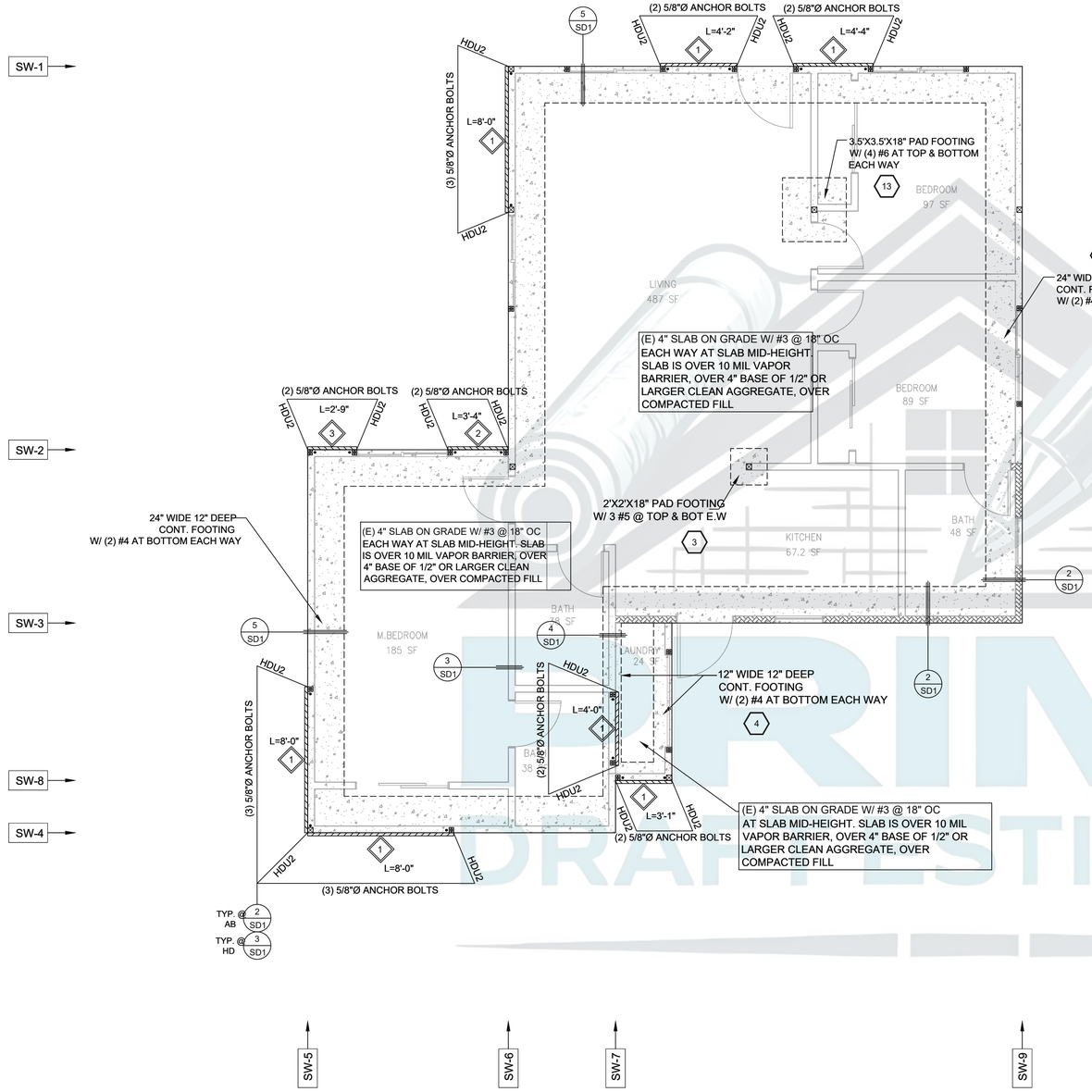
1. USE 2-2xSTUDS OR BLOCKING AT ALL ADJOINING PLYWOOD EDGES.
2. USE "A35" @ 32" O.C. @ ROOF & FLOOR TOP CONNECTION PLATES, U.N.O. BY SHEAR WALL SCHEDULE.
3. ALL STUDS TO BE @ 16" O.C.
4. NAILS SPACED AT 2" O.C. SHALL BE STAGGERED.
5. USE MIN 2x6 STUDS AT PLUMBING WALL(S).
6. THE ANCHOR BOLTS SHALL HAVE AN APPROVED PLATE WASHER UNDER EACH NUT,SEE SCHEDULE AND THE NUT SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING.
7. A.B. TO HAVE 7" EMBEDMENT. DO NOT COUNT HOLDOWNS AS SHEAR ANCHOR BOLTS.
8. USE COMMON NAILS OR GALVANIZED BOX NAILS W/FULL HEADS.
9. 1/2" EDGE DISTANCE FOR PLYWOOD BOUNDARY NAILING.
10. PROVIDE MIN. EDGE DISTANCE OF 1.5 x DIAMETER FOR LAG SCREW BOLTS.
11. PROVIDE MIN. PENETRATION OF 8 x DIAMETER FOR LAG SCREW BOLTS INTO THE HOLDOWN MEMBER.
12. PLYWOOD FIELD NAILING: 10d @ 12" O.C.
13. FOR MORE INFORMATION SEE DETAILS.
14. WOOD STRUCTURE PANEL JOINTS AND SILL PLATE NAILING SHALL BE STAGGERED AT ALL PANEL EDGES WHERE THE DESIGN VALUES OF THE SHEAR WALL EXCEEDS 350 Plf.
15. use of 1/2" OSB STRUCT "1" IS OKAY IN LIEU OF STRUCT "1" PLYWOOD.
16. NON-SHEAR WALL SILL PLATES SHOULD HAVE 16d @ 6" O.C.

BOLT SIZE	WASHER PLATE SIZE
1/2"	1/4"x3"x3"
5/8"	1/4"x3"x3"
3/4"	5/16"x3"x3"
7/8"	5/16"x3"x3"
1"	3/8"x3 1/2"x3 1/2"

SHEAR WALL SCHEDULE | 9

10 | 11 | 12 | 13 | 14 | 15 | 16 | 17

PRIME DRAFT ESTIMATES



FOUNDATION PLAN - BUILDING 01

SCALE: 1/4" = 1'-0"

STRUCTURAL NOTES

1. REFER TO GN-1 & GN-2 SHEETS FOR GENERAL REQUIREMENTS, INCLUDING SHEAR WALL SCHEDULE, SHEATHING, MATERIAL GRADE, ETC.
2. SEE ARCHITECTURAL DRAWING FOR DIMENSIONS NOT SHOWN.
3. ALL FILL MATERIAL IS TO BE COMPACTED TO 90% OF MAXIMUM DENSITY (OR MORE IF REQUIRED IN SOILS REPORT WHERE OCCURS).
4. PRIOR TO CALL FOR FOUNDATION INSPECTION, FINAL GRADING AND COMPACTION REPORTS SHALL BE SUBMITTED TO AND APPROVED BY THE BUILDING DEPARTMENT; AND ALL HOLDOWNS, ANCHOR BOLTS OR OTHER CAST-IN-PLACE FASTENERS SHALL BE SECURED IN PLACE.
5. FASTENERS INSTALLED IN PRESSURE TREATED WOOD SHALL BE DIPPED ZINC COATED, GALVANIZED OR STAINLESS STEEL.
6. HARDWARE CALLED ON PLAN ARE BY SIMPSON STRONG-TIE U.N.O.; INSTALLATION SHALL FOLLOW MANUFACTURER'S REQUIREMENTS WITH MAXIMUM FASTENER AMOUNT, U.N.O.; ALTERNATIVES ARE ACCEPTABLE, PROVIDED CAPACITY IS PROVED TO BE NO LESS THAN SIMPSON STRONG-TIE PRODUCT BY CODE REPORTS.
7. POST-INSTALLED ANCHORS: (N) 5/8"Ø SHEAR ANCHOR BOLTS SHALL BE SET-3G EPOXY ANCHORS WITH 7" MIN. EMBEDMENT. (N) HDU2 HOLD-DOWN ANCHORS SHALL BE 5/8"Ø SET-3G EPOXY ANCHORS WITH 7" MIN. EMBEDMENT. SEE "POST-INSTALLED ADHESIVE ANCHORS" ON GN-01.

LEGEND

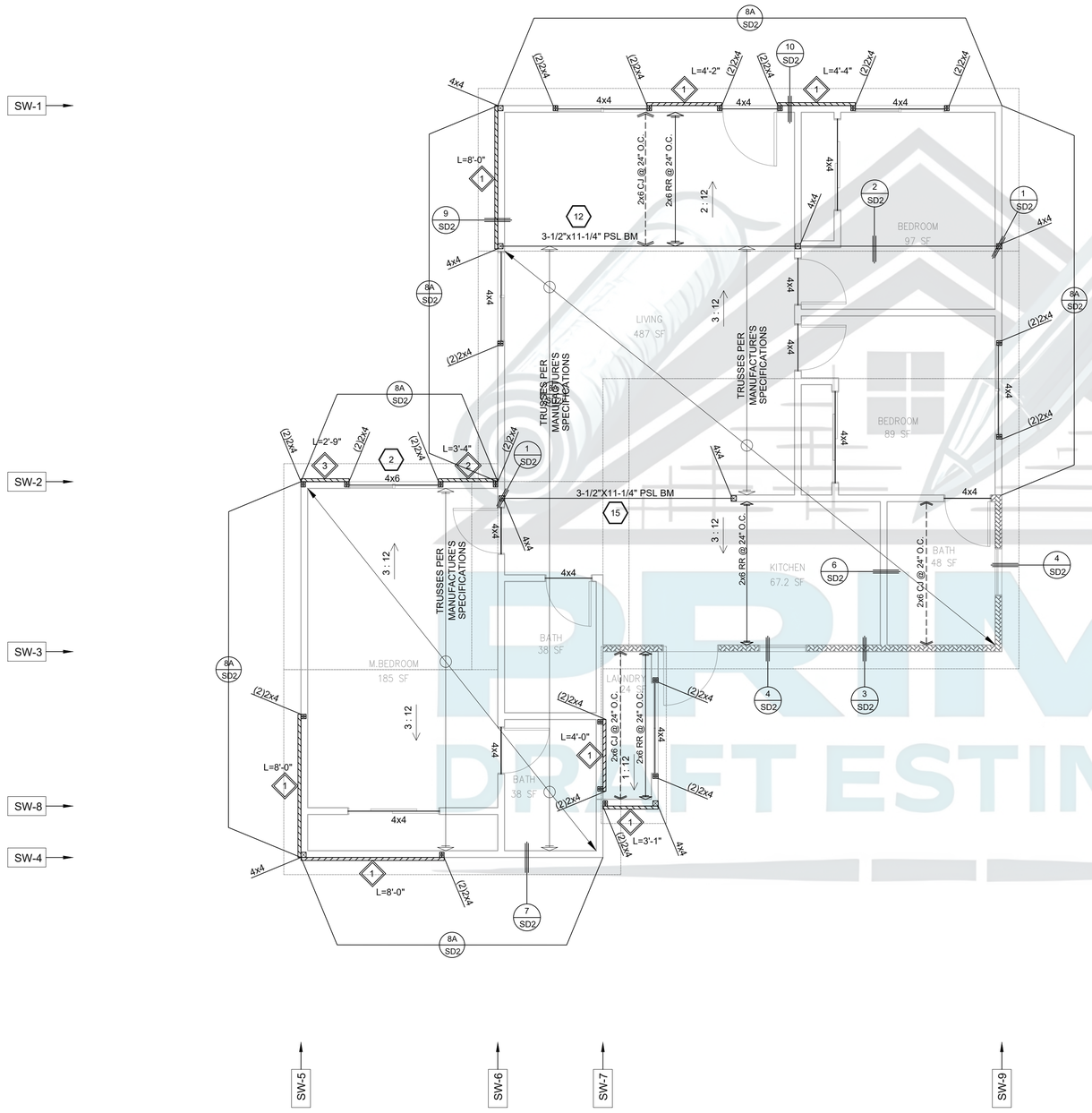
- WOOD POST
- (E) CONCRETE FOOTING
- X / SDX — DETAIL 'X' ON SHEET 'SDX'
- WOOD WALLS
- CMU WALLS
- X — NUMBER 'X' ON CALCULATION TABLE 5.1

TYPICAL DETAILS

- 3 / GN-2 — TYPICAL REINFORCING AND BAR BEND & SPLICE
- 5 / GN-2 — TYPICAL SLAB ON GRADE
- 1 / SD-1 — TYPICAL CONCRETE REINFORCEMENT

SLAB / FOUNDATION NOTE:
EXISTING SLAB-ON-GRADE TO REMAIN. NO NEW SLAB IS PROPOSED. NO NEW EXCAVATION OR NEW EXTERIOR FOOTING INSULATION IS REQUIRED. REPAIRS ONLY.

PRIME DRAFT ESTIMATES



FRAMING PLAN - BUILDING 01

SCALE: 1/4" = 1'-0"

STRUCTURAL NOTES

1. REFER TO GN-1 & GN-2 SHEETS FOR GENERAL REQUIREMENTS, INCLUDING SHEAR WALL SCHEDULE, SHEATHING, MATERIAL GRADE, ETC.
2. SEE ARCHITECTURAL DRAWING FOR DIMENSIONS NOT SHOWN.
3. FASTENERS INSTALLED IN PRESSURE TREATED WOOD SHALL BE DIPPED ZINC COATED, GALVANIZED OR STAINLESS STEEL.
4. HARDWARE CALLED ON PLAN ARE BY SIMPSON STRONG-TIE U.N.O.; INSTALLATION SHALL FOLLOW MANUFACTURER'S REQUIREMENTS WITH MAXIMUM FASTENER AMOUNT, U.N.O.; ALTERNATIVES ARE ACCEPTABLE, PROVIDED CAPACITY IS PROVED TO BE NO LESS THAN SIMPSON STRONG-TIE PRODUCT BY CODE REPORTS.
5. ALL TOP PLATES SHALL BE SPLICED WITH 6A/SD1 U.N.O.
6. USE SIMPSON IUS/ITS/U HANGERS FOR CONNECTION OF I-JOIST U.N.O.; USE SIMPSON LUS/U HANGERS FOR CONNECTION OF SOILID JOISTS U.N.O.

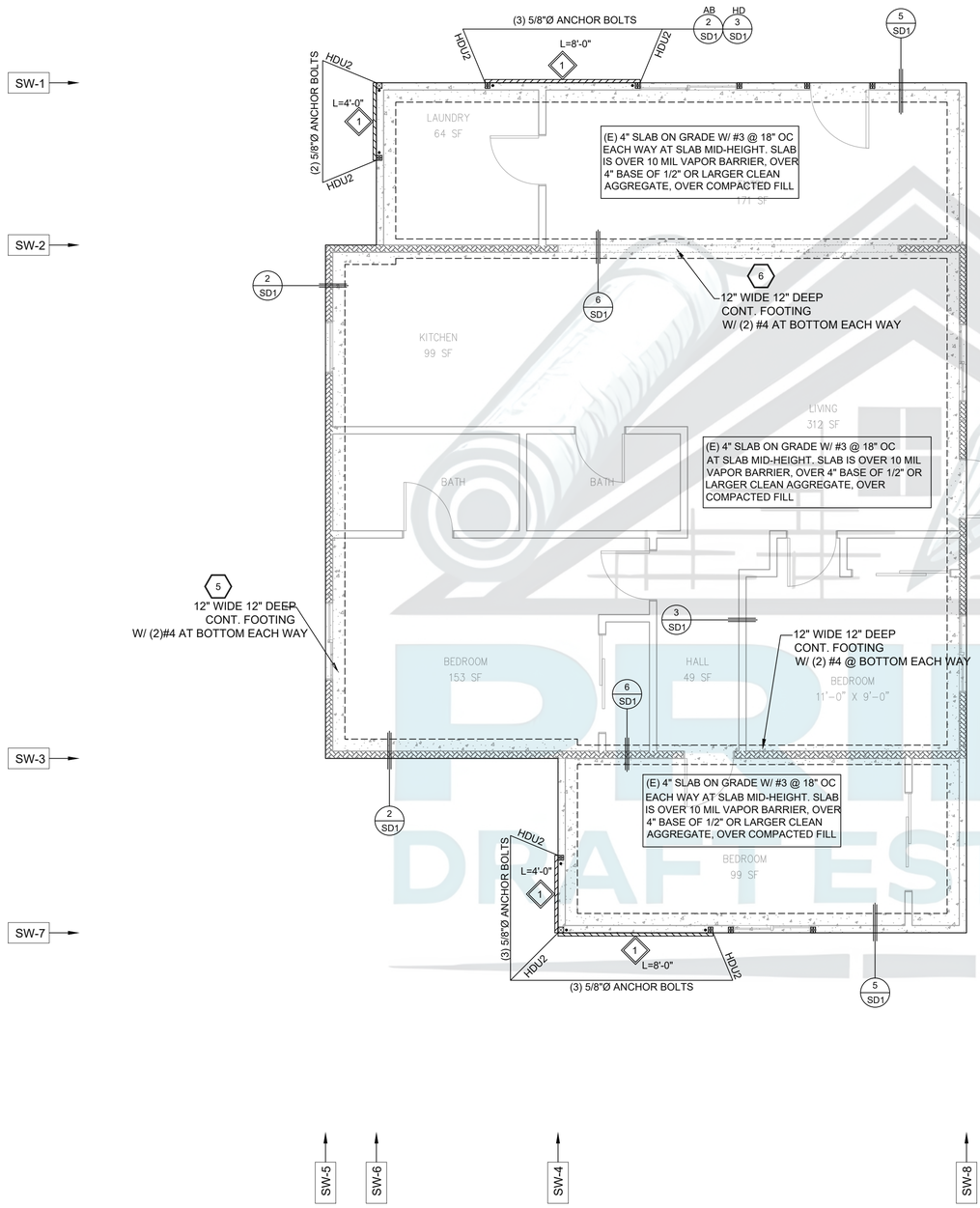
LEGEND

	WOOD POST
X / SDX	DETAIL 'X' ON SHEET 'SDX'
	WOOD WALLS
	CMU WALLS
X	NUMBER 'X' ON CALCULATION TABLE 5.1

TYPICAL DETAILS

1 / GN-2	TYPICAL DRILLING AND NOTCHING OF WOOD JOISTS
2 / GN-2	TYPICAL DRILLING AND NOTCHING OF WOOD STUDS
4 / GN-2	TYPICAL WALL ELEVATION AND HEADER SCHEDULE
6 / GN-2	TYPICAL STUD CONFIGURATION PLAN
7 / GN-2	TYPICAL PANEL LAYOUT
9 / GN-2	TYPICAL SHEAR WALL SCHEDULE

PRIME DRAFT ESTIMATES



STRUCTURAL NOTES

1. REFER TO GN-1 & GN-2 SHEETS FOR GENERAL REQUIREMENTS, INCLUDING SHEAR WALL SCHEDULE, SHEATHING, MATERIAL GRADE, ETC.
2. SEE ARCHITECTURAL DRAWING FOR DIMENSIONS NOT SHOWN.
3. ALL FILL MATERIAL IS TO BE COMPACTED TO 90% OF MAXIMUM DENSITY (OR MORE IF REQUIRED IN SOILS REPORT WHERE OCCURS).
4. PRIOR TO CALL FOR FOUNDATION INSPECTION, FINAL GRADING AND COMPACTION REPORTS SHALL BE SUBMITTED TO AND APPROVED BY THE BUILDING DEPARTMENT; AND ALL HOLDOWNS, ANCHOR BOLTS OR OTHER CAST-IN-PLACE FASTENERS SHALL BE SECURED IN PLACE.
5. FASTENERS INSTALLED IN PRESSURE TREATED WOOD SHALL BE DIPPED ZINC COATED, GALVANIZED OR STAINLESS STEEL.
6. HARDWARE CALLED ON PLAN ARE BY SIMPSON STRONG-TIE U.N.O.; INSTALLATION SHALL FOLLOW MANUFACTURER'S REQUIREMENTS WITH MAXIMUM FASTENER AMOUNT, U.N.O.; ALTERNATIVES ARE ACCEPTABLE, PROVIDED CAPACITY IS PROVED TO BE NO LESS THAN SIMPSON STRONG-TIE PRODUCT BY CODE REPORTS.
7. POST-INSTALLED ANCHORS: (N) 5/8"Ø SHEAR ANCHOR BOLTS SHALL BE SET-3G EPOXY ANCHORS WITH 7" MIN. EMBEDMENT. (N) HDU2 HOLD-DOWN ANCHORS SHALL BE 5/8"Ø SET-3G EPOXY ANCHORS WITH 7" MIN. EMBEDMENT. SEE "POST-INSTALLED ADHESIVE ANCHORS"

LEGEND

- WOOD POST
- (E) CONCRETE FOOTING
- X / SDX — DETAIL 'X' ON SHEET 'SDX'
- WOOD WALLS
- CMU WALLS
- X — NUMBER 'X' ON CALCULATION TABLE 5.1

TYPICAL DETAILS

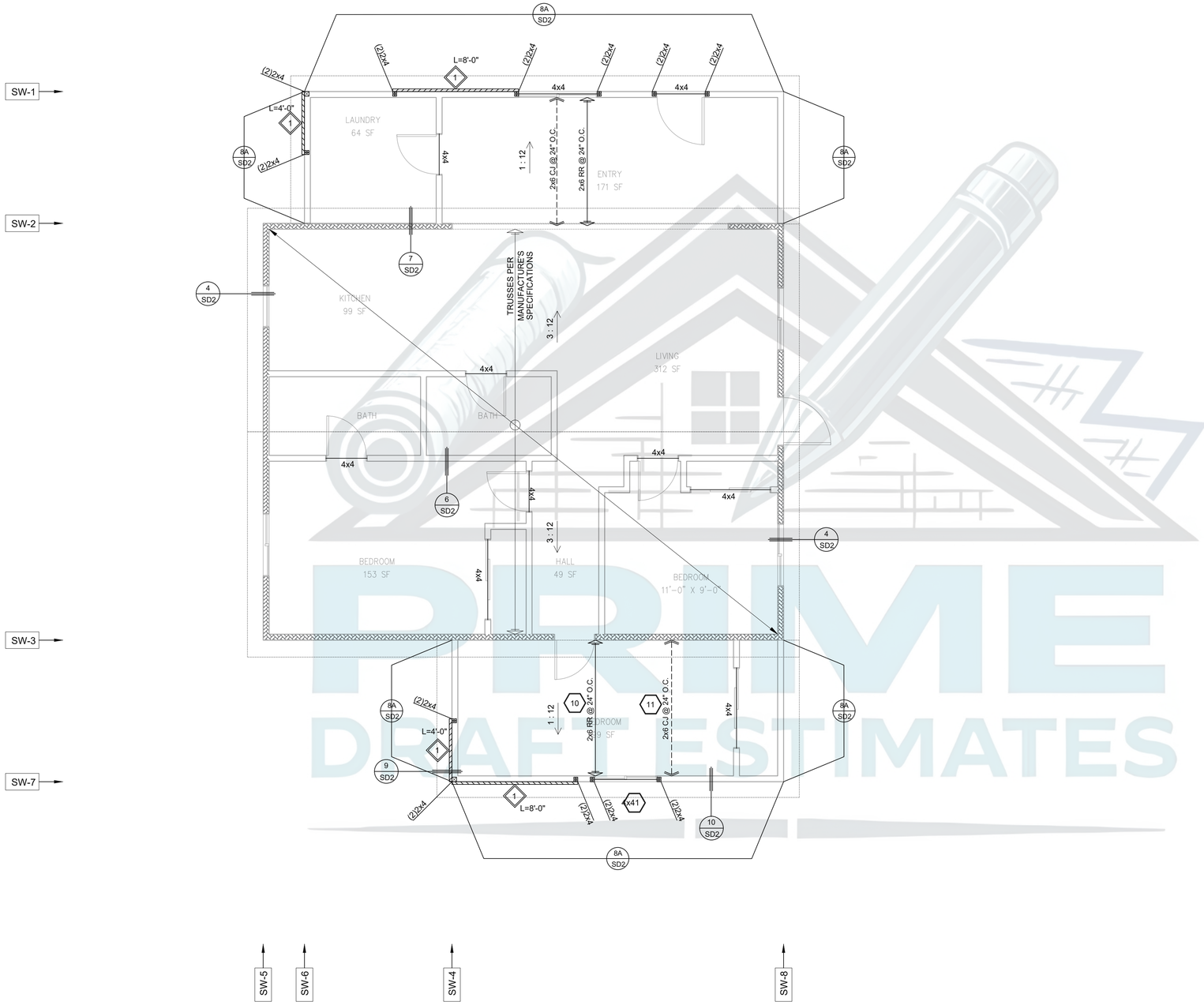
- 3 / GN-2 — TYPICAL REINFORCING AND BAR BEND & SPLICE
- 5 / GN-2 — TYPICAL SLAB ON GRADE
- 1 / SD-1 — TYPICAL CONCRETE REINFORCEMENT

SLAB / FOUNDATION NOTE:
EXISTING SLAB-ON-GRADE TO REMAIN. NO NEW SLAB IS PROPOSED. NO NEW EXCAVATION OR NEW EXTERIOR FOOTING INSULATION IS REQUIRED. REPAIRS ONLY.

FOUNDATION PLAN - BUILDING 02

SCALE: 1/4" = 1'-0"

PRIME DRAFT ESTIMATES



FRAMING PLAN - BUILDING 02

SCALE: 1/4" = 1'-0"

STRUCTURAL NOTES

1. REFER TO GN-1 & GN-2 SHEETS FOR GENERAL REQUIREMENTS, INCLUDING SHEAR WALL SCHEDULE, SHEATHING, MATERIAL GRADE, ETC.
2. SEE ARCHITECTURAL DRAWING FOR DIMENSIONS NOT SHOWN.
3. FASTENERS INSTALLED IN PRESSURE TREATED WOOD SHALL BE DIPPED ZINC COATED, GALVANIZED OR STAINLESS STEEL.
4. HARDWARE CALLED ON PLAN ARE BY SIMPSON STRONG-TIE U.N.O.; INSTALLATION SHALL FOLLOW MANUFACTURER'S REQUIREMENTS WITH MAXIMUM FASTENER AMOUNT, U.N.O.; ALTERNATIVES ARE ACCEPTABLE, PROVIDED CAPACITY IS PROVED TO BE NO LESS THAN SIMPSON STRONG-TIE PRODUCT BY CODE REPORTS.
5. ALL TOP PLATES SHALL BE SPLICED WITH 6A/SD1 U.N.O.
6. USE SIMPSON IUS/ITS/U HANGERS FOR CONNECTION OF I-JOIST U.N.O.; USE SIMPSON LUS/U HANGERS FOR CONNECTION OF SOILID JOISTS U.N.O.

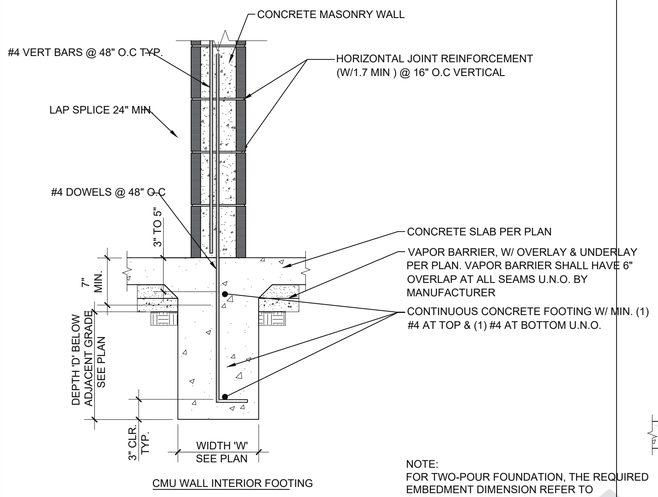
LEGEND

- WOOD POST
- X / SDX — DETAIL 'X' ON SHEET 'SDX'
- WOOD WALLS
- CMU WALLS
- X — NUMBER 'X' ON CALCULATION TABLE 5.1

TYPICAL DETAILS

- 1 / GN-2 — TYPICAL DRILLING AND NOTCHING OF WOOD JOISTS
- 2 / GN-2 — TYPICAL DRILLING AND NOTCHING OF WOOD STUDS
- 4 / GN-2 — TYPICAL WALL ELEVATION AND HEADER SCHEDULE
- 6 / GN-2 — TYPICAL STUD CONFIGURATION PLAN
- 7 / GN-2 — TYPICAL PANEL LAYOUT
- 9 / GN-2 — TYPICAL SHEAR WALL SCHEDULE

PRIME DRAFT ESTIMATES

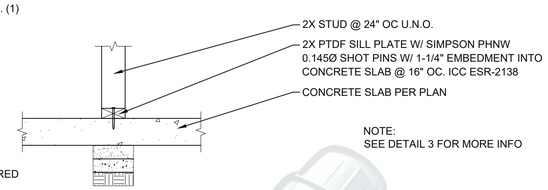


CMU WALL INTERIOR FOOTING

NOTE:
FOR TWO-POUR FOUNDATION, THE REQUIRED EMBEDMENT DIMENSION REFER TO EMBEDMENT INTO FIRST POUR U.N.O.

	9	CMU WALL INTERIOR FOUNDATION DETAIL	6	INTERIOR NON-BEARING WALL	3

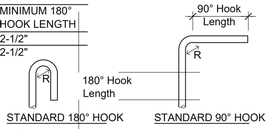
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NOTE:
SEE DETAIL 3 FOR MORE INFO

STANDARD HOOK DIMENSIONS, "L EXT"

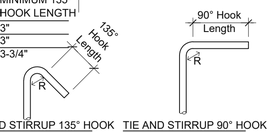
BAR SIZE	R	MINIMUM 90° HOOK LENGTH	MINIMUM 180° HOOK LENGTH
#4	3"	6"	2-1/2"
#5	3-3/4"	7-1/2"	2-1/2"



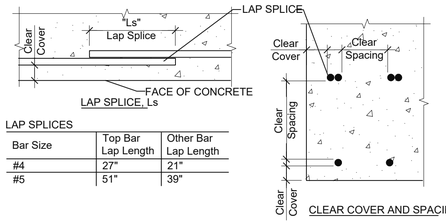
STANDARD 180° HOOK STANDARD 90° HOOK

TIE AND STIRRUP HOOK DIMENSIONS, "L EXT"

BAR SIZE	R	MINIMUM 90° HOOK LENGTH	MINIMUM 135° HOOK LENGTH
#3	1-1/2"	3"	3"
#4	2"	3"	3"
#5	2-1/2"	3-3/4"	3-3/4"



TIE AND STIRRUP 135° HOOK TIE AND STIRRUP 90° HOOK



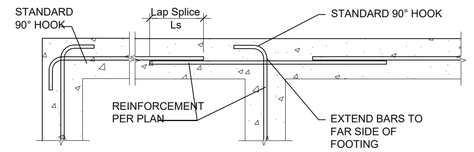
CLEAR COVER AND SPACING

LAP SPLICES

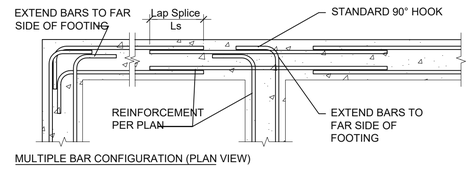
Bar Size	Top Bar Lap Length	Other Bar Lap Length
#4	27"	21"
#5	51"	39"

NOTE:
1. THIS SCHEDULE SHALL BE USED WHERE DEVELOPMENT AND SPLICE LENGTHS ARE NOT SPECIFICALLY DETAILED AND DIMENSIONED ON THE DRAWINGS OR IN OTHER DETAILS.
2. FY=60 KSI EXCEPT #3 BARS fy = 40 KSI.
3. "TOP" REINFORCEMENT IS DEFINED AS HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF FRESH VERTICAL BARS AND ANY OTHER HORIZONTAL BARS.

LENGTHS OF THE LARGER BAR, OR SPLICE LENGTH OF SMALLER BAR.
5. LAP SPLICES ARE ASSUMED CLASS B.



SINGLE BAR CONFIGURATION (PLAN VIEW)



MULTIPLE BAR CONFIGURATION (PLAN VIEW)

INTERIOR FOOTING

NOTE:
FOR TWO-POUR FOUNDATION, THE REQUIRED EMBEDMENT DIMENSION REFER TO EMBEDMENT INTO FIRST POUR U.N.O.

	10		7	INTERIOR FOUNDATION DETAIL FN003	4 TYPICAL CONCRETE FOUNDATION 1

EXTERIOR FOOTING

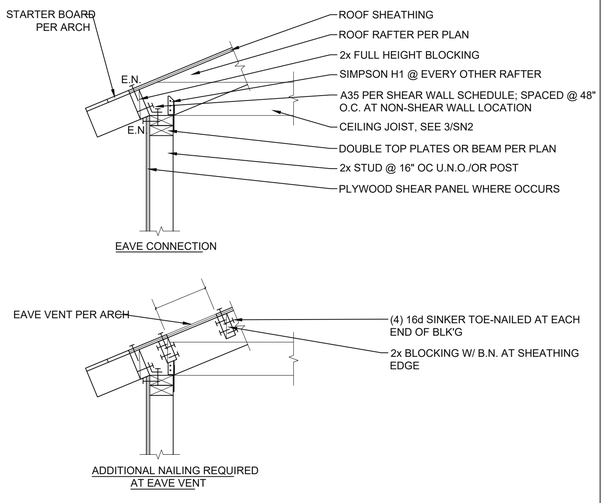
NOTE:
FOR TWO-POUR FOUNDATION, THE REQUIRED EMBEDMENT DIMENSION REFER TO EMBEDMENT INTO FIRST POUR U.N.O.

EXTERIOR FOOTING

NOTE:
FOR TWO-POUR FOUNDATION, THE REQUIRED EMBEDMENT DIMENSION REFER TO EMBEDMENT INTO FIRST POUR U.N.O.

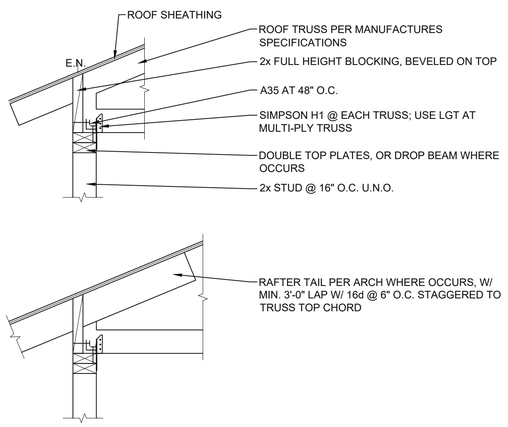
	11		8	CONTINUOUS FOUNDATION DETAIL FN006	5 CONTINUOUS FOUNDATION DETAIL FN006 2

PRIME DRAFT ESTIMATES



RAFTER TO WALL DETAIL

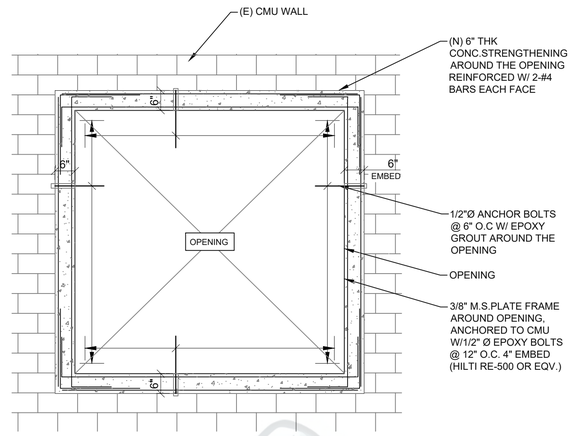
10



WALL & ROOF TRUSS CONNECTION

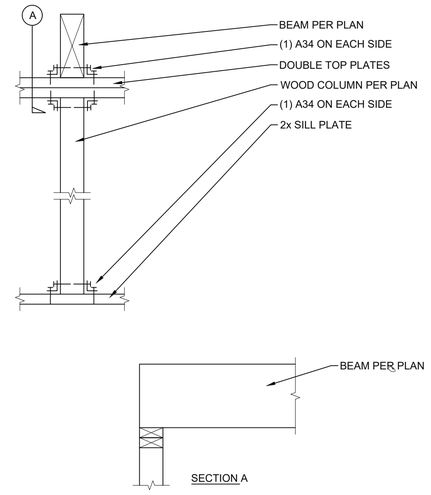
WD029

7



ACCESS OPENING IN CMU

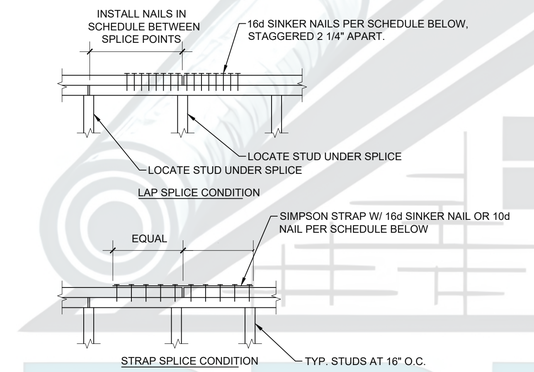
4



BEAM DETAIL

WD001

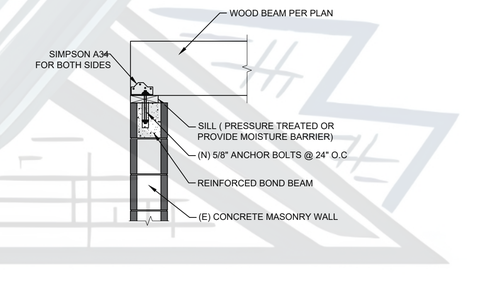
1



	A	B	C	D	E	F	G
NUMBER OF 16d SINKER WITHIN MIN. LAP LENGTH OF PLATE	10 4'	16 4'	22 6'	24 6'	32 8'	36 8'	42 8'
STRAP FULLY NAILED W/ 16d SINKERS	ST22	ST6224	ST6236	MST37	MSTC66	MST60	CMST12 (78")
CAPACITY	1420#	2540#	3845#	5080#	5860#	6730#	7930#

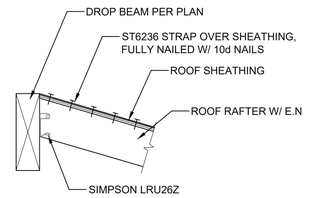
TOP PLATE SPLICE

8



BEAM TO WALL CONNECTION DETAIL

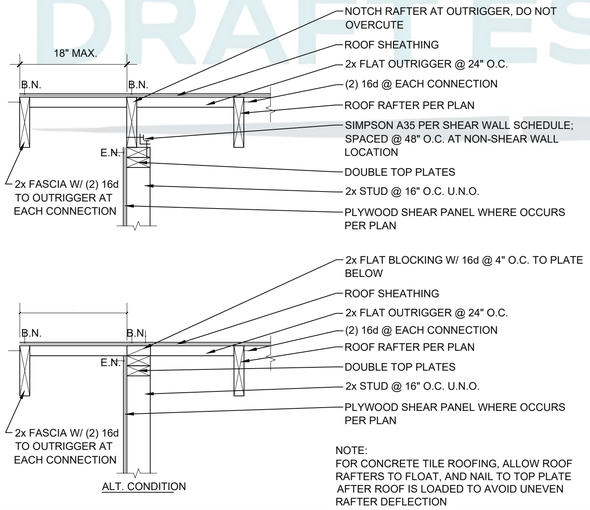
5



RAFTER TO BEAM CONNECTION

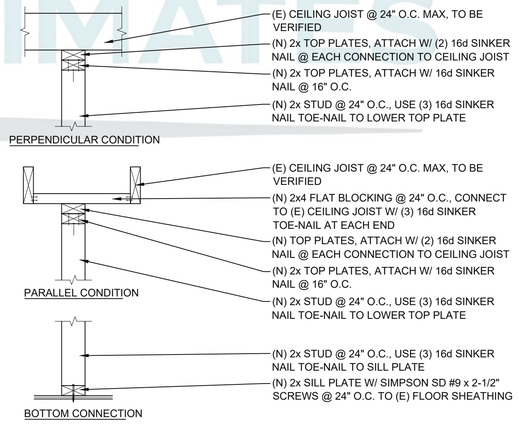
WD055

2



SHEAR TRANSFER

9

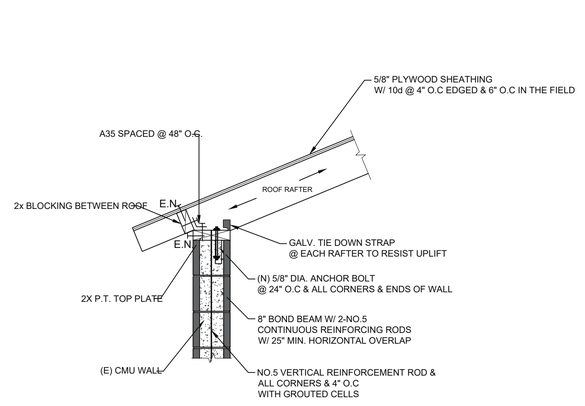


NOTE:
DESIGNED FOR 5 psf WIND FORCE ON 10' TALL WALL, DFL#2 STUD SPACED @ 24" O.C.:
0.6 x 5 psf x 10' x 2' x 1/2 = 30 lbs AT EACH CONNECTION ON TOP OR BOTTOM
(3) 16d SINKER TOE NAIL LATERAL CAPACITY IS 3 x 103 x 0.67 = 207 lbs
SIMPSON SD #9 x 2-1/2" SCREW LATERAL CAPACITY IS 133 lbs

INTERIOR NON-BEARING WALL

WD023

6



ROOF RAFTER TO CMU WALL CONNECTION DTL

3

PRIME DRAFT ESTIMATES